

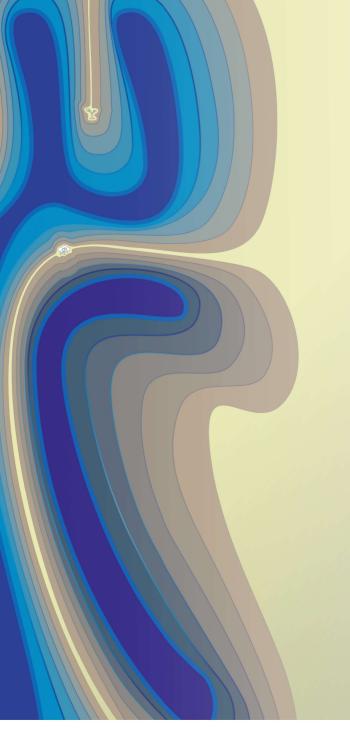
Risk zoning of huge flood based on coupled hydrodynamic model

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第18届 世界水资源大会 **57***





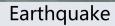
Content

- Introduction
- Study area and methodology
- Model construction
- Results and discussion
- Summarize

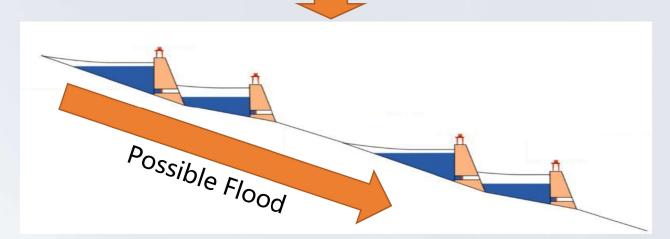








Huge flood



Cascade reservoirs



The breach of South Fork dam



The breach of Kahovka dam

Large-scale earthquakes and unusual floods can cause the failure of dams, resulting in massive floods. This may trigger the subsequent collapse of cascade reservoirs.



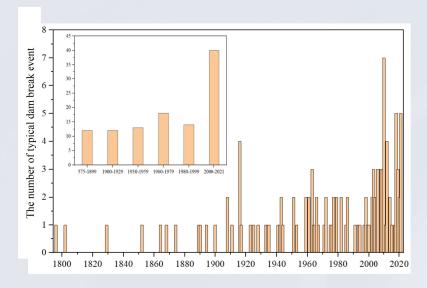


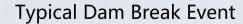
Global warming

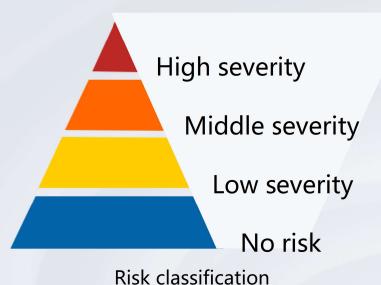




Extreme hydroclimatic



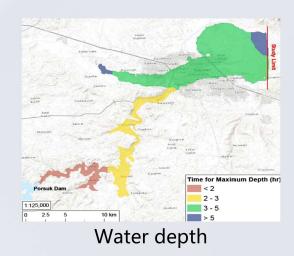


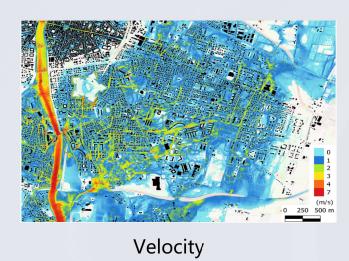


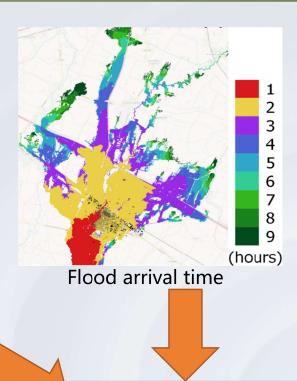
The global warming amplified the hydroclimatic extremes and increased their frequency. Furthermore, the typical dam break events increase, as time goes by since 1800. Therefore, the **risk zoning** of dam break floods has emerged as a challenging task for researchers.



□ Risk zoning based on flood parameters







Those risk zoning methods based on flood parameters can indicate the flood path, they only reveal its characteristics, not how it affects people and cities.

What about the effects on people and cities?



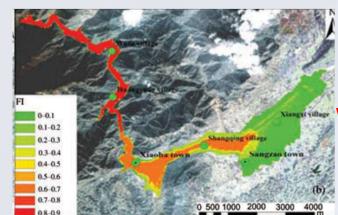


□ Risk zoning based on evaluation system

$$FI = 0.45WD + 0.22WR + 0.22V + 0.11D$$

$$WD = 0.665 \times 10^{-3} e^{1.16h_{max}}$$

 $Disaster = PD \times FI$



Flood-exposure index (FI)

Those risk zoning methods based on evaluation system can indicate the **risk spatial distribution** and the risk level of whole flood, but don't consider the **personal evacuation** which considerably impacts risk zoning.



Spatial flood risk assessment

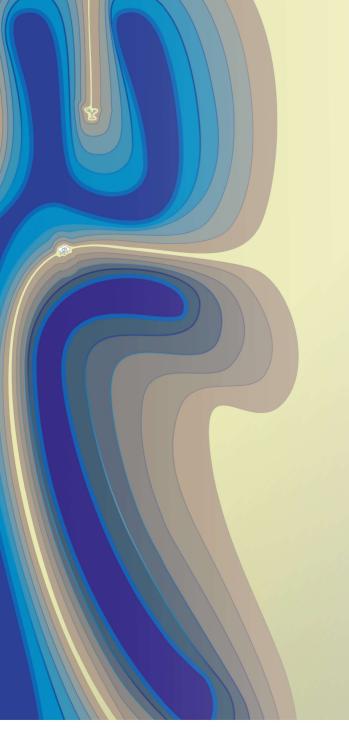




Rescue operation

The **life safety** in floods is the most important. So there need to be a risk zoning **considering personnel evacuation**. That could show who need to be evacuation and who need to find a emergency shelter.





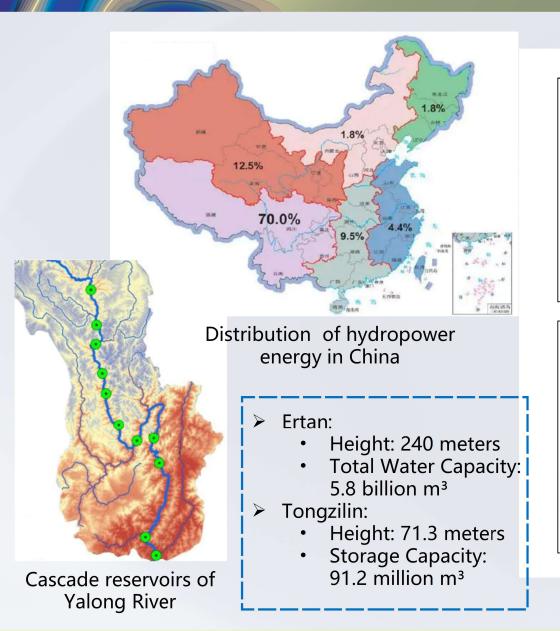


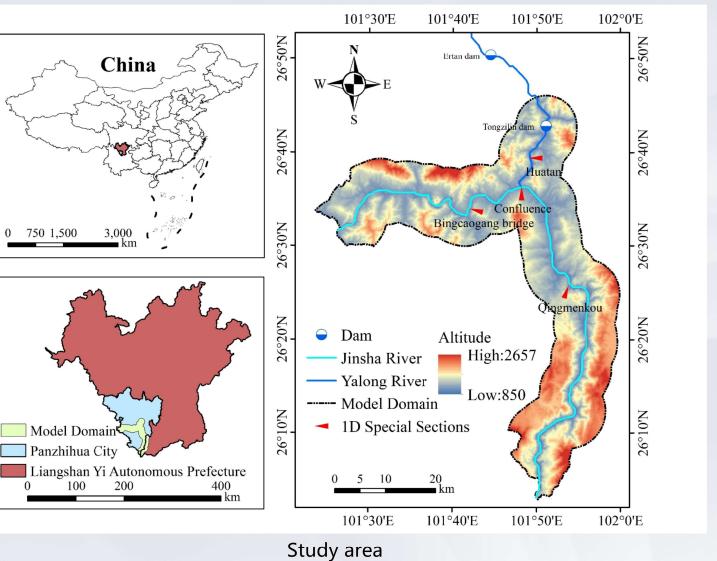
Content

- Introduction
- Study area and methodology
- Model construction
- Results and discussion
- Summarize

2.1 Study area







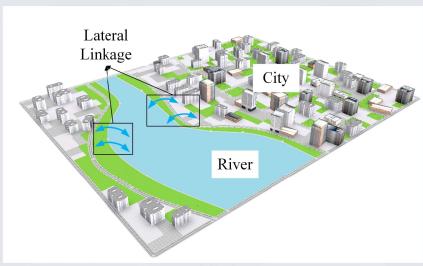
2.2 Coupled hydrodynamic model



□ coupled hydrodynamic model



Overflow



Lateral linkages

The lateral coupling model is employed to simulate the flood spread in river (1D model) and in surface (2D model). The exchange flow between 1D and 2D model is calculated by weir equation.

□ Governing equation

$$\frac{\partial Q}{\partial x} + \frac{\partial A}{\partial t} = 0$$

$$gA\left(\frac{\partial Z}{\partial x} + S_f\right) + \frac{\partial (Q^2/A)}{\partial x} + \frac{\partial Q}{\partial t} = 0$$

$$\frac{\partial H}{\partial t} + \frac{\partial (Hu)}{\partial x} + \frac{\partial (Hv)}{\partial y} = S_{ce}$$

$$\frac{\partial u}{\partial t} + u \frac{\partial u}{\partial x} + v \frac{\partial u}{\partial y} = -g \frac{\partial z}{\partial x} + F_x + v_e (\frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 u}{\partial y^2})$$

$$\frac{\partial v}{\partial t} + u \frac{\partial v}{\partial x} + v \frac{\partial v}{\partial y} = -g \frac{\partial z}{\partial x} + F_y + v_e (\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial x^2})$$

□ Weir equation

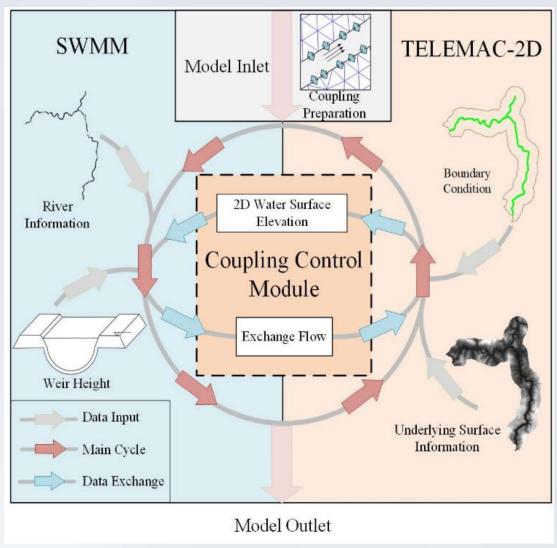
$$Q = \begin{cases} 0.35bh_{max}\sqrt{2gh_{max}} & if \frac{h_{min}}{h_{max}} \leq \frac{2}{3} \\ 0.91bh_{min}\sqrt{2g(h_{max} - h_{min})} & if \frac{2}{3} < \frac{h_{min}}{h_{max}} \leq 1 \end{cases}$$

$$h_{max} = max(H_r, H_s) - Z$$

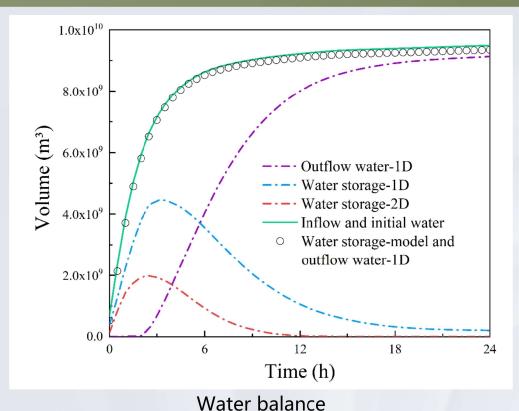
$$h_{min} = min(H_r, H_s) - Z$$

2.2 Coupled hydrodynamic model





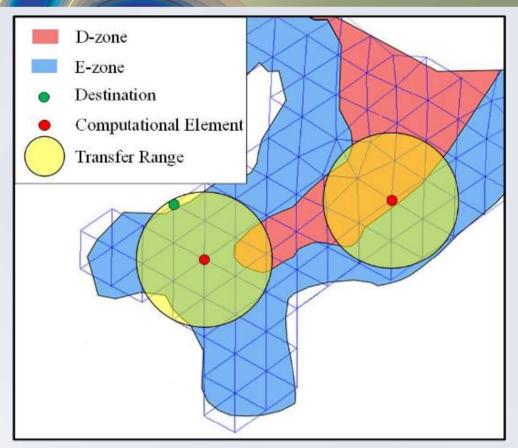
Coupling model operation progress

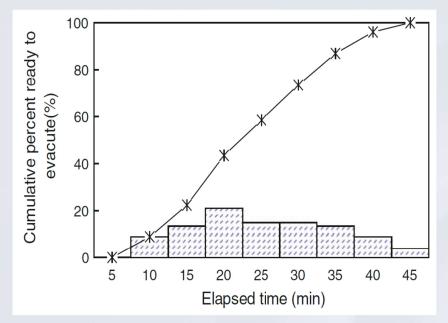


- Step1 Coupling preparetion:
- Step2 Operation of the 1D module
- Step3 Operation of the 2D module
- Step4 Process control

2.3 Risk zoning method







Evaluate probability density distribution of generation time

$$t_T = t_d + t_w + t_p + t_e$$

$$t_{EGT} = t_w + t_p$$

$$t_d = 0$$

Composition of evaluation generation time (EGT)

Risk zoning method



D-zone



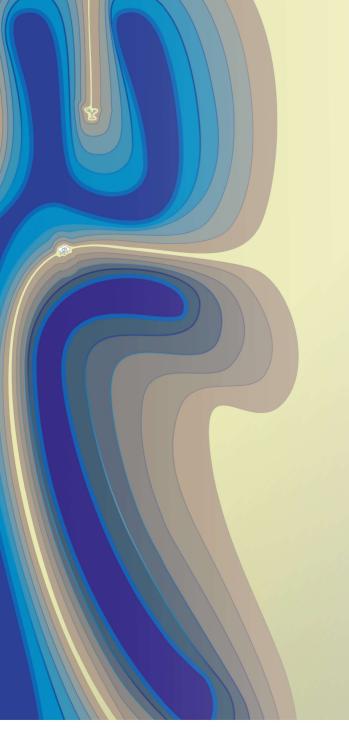
E-zone



People cannot leave before the floodwaters come

People have sufficient time to evacuate

- Step1 Identify computational elements
- Step2 Determine the transfer speed and evaluation generation time (EGT)
- Step3 Calculate the transfer range
- Step4 Identify D-zone and E-zone



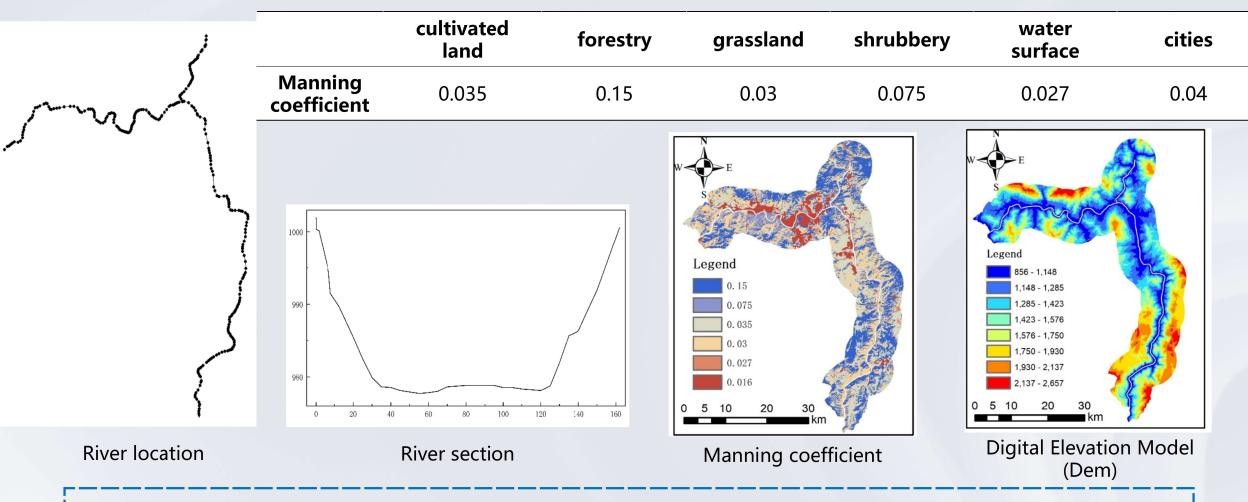


Content

- Introduction
- Study area and methodology
- Model construction
- Results and discussion
- Summarize

3.1 Model construction

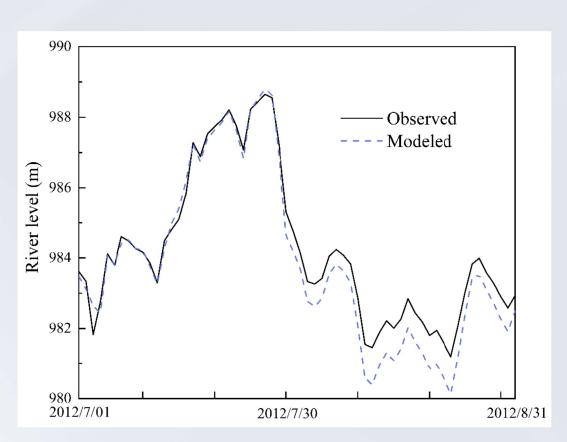




The 137 km long river in the 1D module was split into multiple short sections. Moreover, a surface of 1506 km² in the 2D module was covered with an unstructured triangular mesh with a resolution of 30 m.

3.2 Model calibration and validation





Model validation by river level

$$r = \frac{\sum_{i=1}^{n} (h_{m}^{i} - \bar{h}_{m})(h_{o}^{i} - \bar{h}_{o})}{\sqrt{\sum_{i=1}^{n} (h_{m}^{i} - \bar{h}_{m})^{2}} \sqrt{\sum_{i=1}^{n} (h_{o}^{i} - \bar{h}_{o})^{2}}}$$

$$NSE = 1 - \frac{\sum_{i=1}^{n} (h_{m}^{i} - h_{o}^{i})^{2}}{\sum_{i=1}^{n} (h_{o}^{i} - \bar{h}_{o})^{2}}$$

This study describes the Manning coefficient of the river channel range obtained from the barrier lake break flood of Tanggudong on the Yalong River. In the range of 0.045–0.065, the Manning coefficient was validated by the observed river level. As expected, the model performed well with Pearson correlation coefficient (r) of 0.99 and Nash–Sutcliffe Efficiency (NSE) of 0.93.

3.3 Manning coefficient





Flood destroyed buildings



Flood destroyed buildings

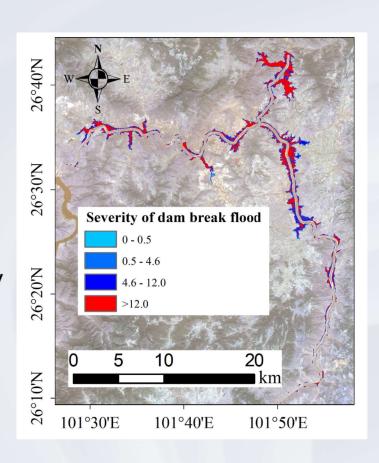
□ Building risk

 $DV = V \times H$ V is velocity, m/s; H represents water depth, m_{\bullet}

 $DV < 4.6 \ m^2/s$ Low severity $4.6 \ m^2/s \le DV < 12 \ m^2/s$ Middle severity $DV \ge 12 \ m^2/s$ High severity

In this case, a reduced

Manning coefficient was used to characterize the water spread following a building collapse in building areas.

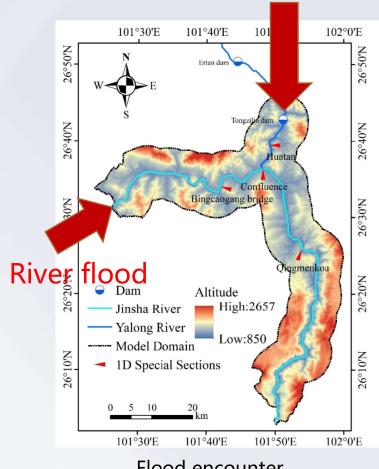


Distribute of building risk

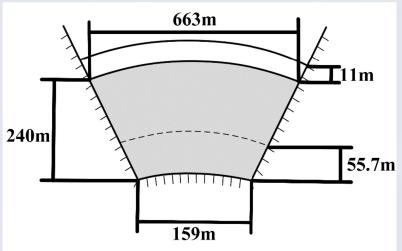
3.4 Flood scenarios







Flood encounter



Ertan dam section $1.0x10^{10}$ 1.0×10^6 8.0×10^9 Discharge (m³/s) 8.0x10⁵ 6.0x10⁵ 4.0x10⁵ 6. 0x10⁹ E Discharge progress Accumulated water 4. 0x10⁹ N 2.0×10^9 $2.0x10^5$ **1** 0. 0 0.0 12 16 20 Time(h) Dam-break flood hydrograph

Dam break flood

Scenario 1 FLOOD (S1)Annual average flow



Scenario 2 (S2)

3.4 Rise zoning scenarios





Scenario1

Scenario 1-30

(S1-30)



Scenario2

Scenario 2-30

(S2-60)

Two flood scenarios and two different Evaluation Generation Time (EGT) are combined to form **four risk zoning scenarios**. The two EGT is ,**30 min and 60 min** respectively. The detailes of the scenarios is shown in the table.



EGT=30min

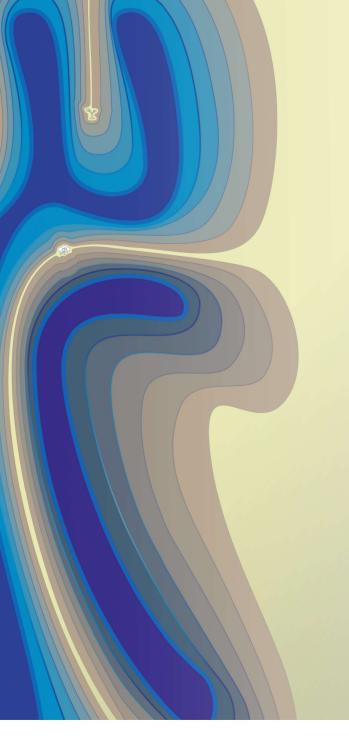
Scenario 1-60

EGT=60min (S1-30)

Scenario 2-60

(S2-60)

Flood scenarios	Evacuation generation times	
	30 min	60 min
Scenario 1 (S1)	Scenario 1-30 (S1-30)	Scenario 1-60 (S1-60)
Scenario 2 (S2)	Scenario 2-30 (S2-30)	Scenario 2-60 (S2-60)



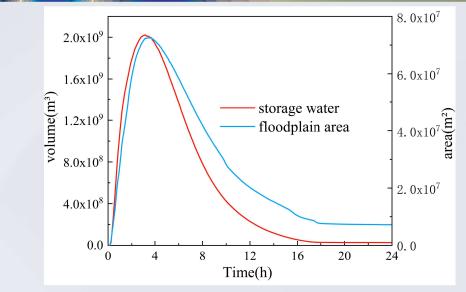


Content

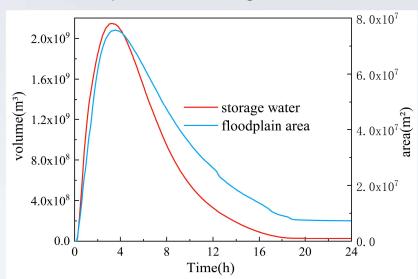
- Introduction
- Study area and methodology
- Model construction
- Results and discussion
- Summarize

4.1 Flood routing

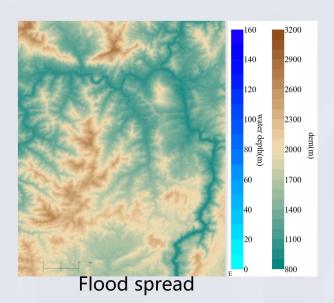




Floodplain and storage water (S1)



Floodplain and storage water (S2)

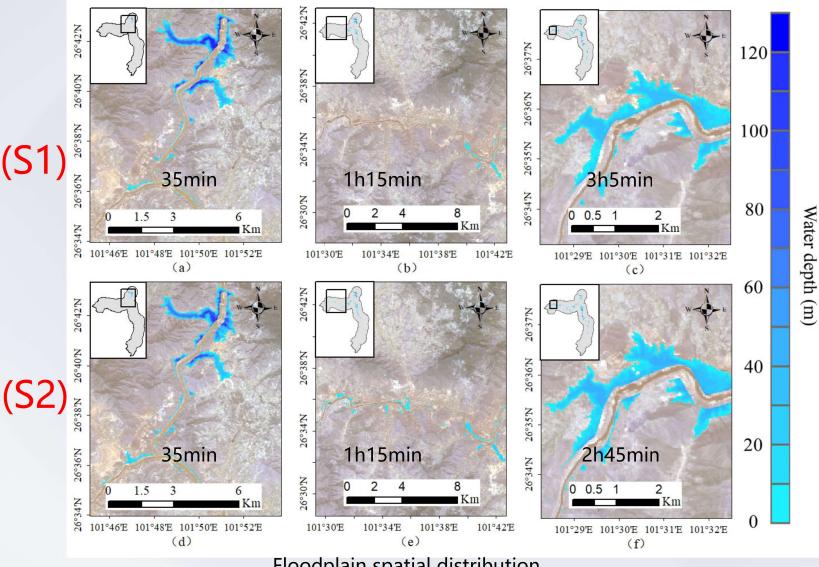


At the junction, the flood will **spread upstream** and down stream along the Jinsha River.

The hydrograph of the floodplain is similar to the water storage capacity in the 2D model. But the water storage capacity is close to zero after 18 hours, while the floodplain is nearly 7 km². This is because some have stored water that cannot flow back into the river. And the water depth in these areas is so shallow that the water storage approaches zero.

4.1 Flood routing



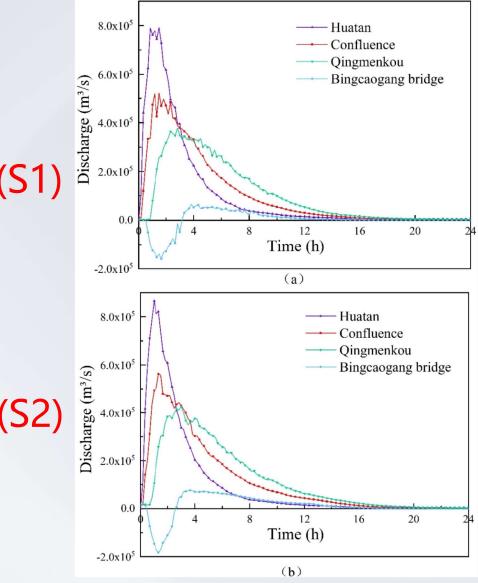


All flood scenarios had a nearly identical floodplain in the first 35 min. While the floodplain began to be different as it reached the Jinsha River. In scenario1 (S1) had a floodplain of 43.42 km² in 1 h 20 min. In contrast, scenario2 (S2), the flood inundated 47.34 km² in 1 h 20 min. The Jinsha River dam-break flood had varying velocities, which caused difference in the damage.

Floodplain spatial distribution

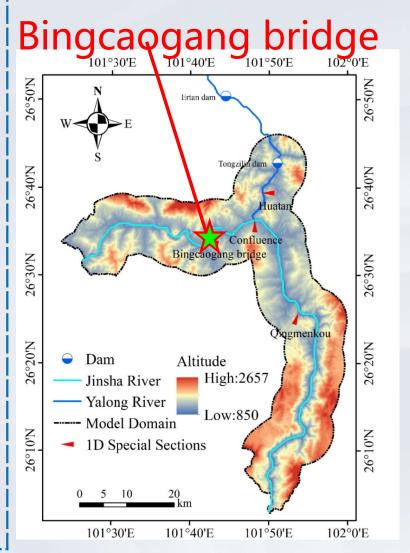
4.2 River section discharge





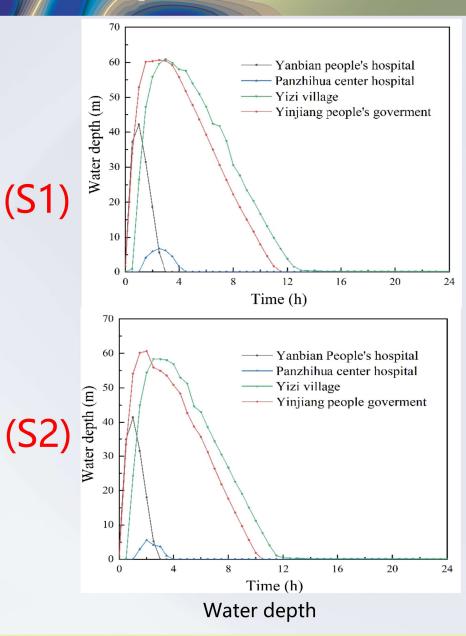
Discharge of river section

The water level variation at the **Bingcaogang Bridge** was unusual. The discharge at the Bingcaogang Bridge abruptly became **negative** shortly after the dam broke, indicating the arrival of the dam-break flood. The difference of the peak flow was 14.42%, when the difference in countercurrent duration across the scenarios was 13.33%.

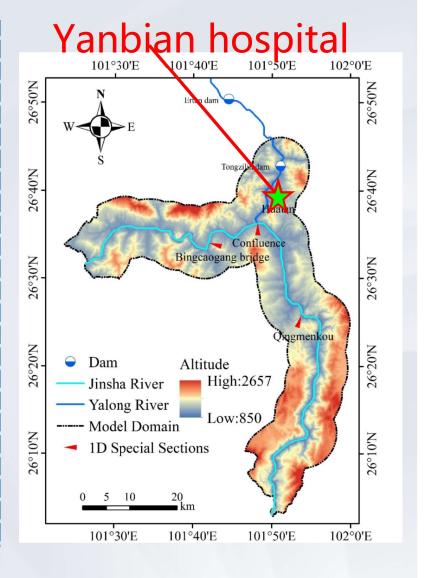


4.3 Special node water depth



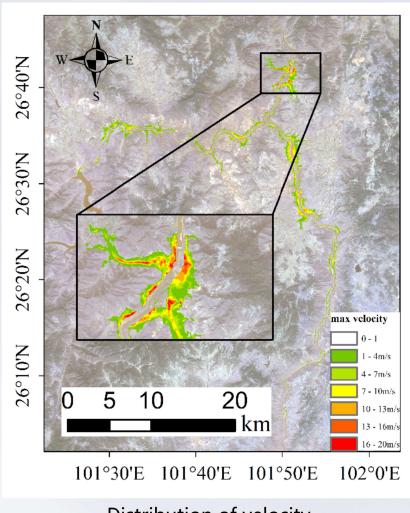


As the distance from Tongzilin increases, the peak water depth in each section decreases. However, the **depth of special points** is different for the different elevations. Yanbian Hospital closes to Tongzilin Reservoir and located on the bank of Yalong River, so the **rising** and falling of water depth in there is relatively earlier.

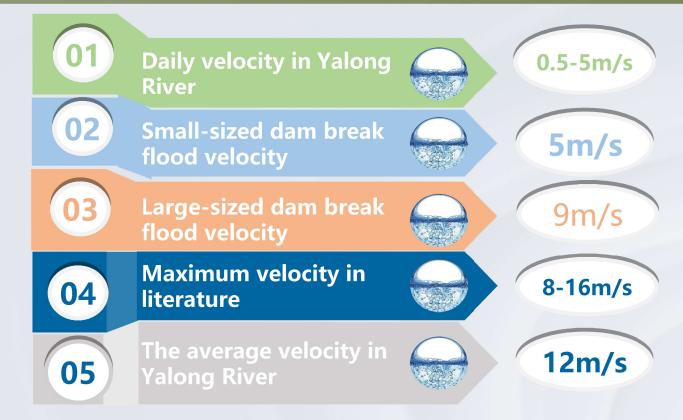


4.4 Flood velocity





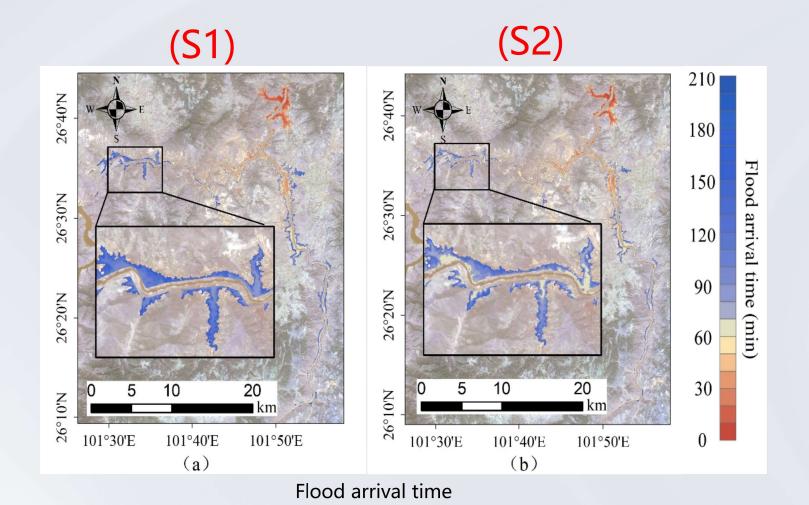
Distribution of velocity



The flood velocity along the Yalong River is higher than other regions. And the average velocity in there is 12m/s. That is higher than other researches for the huge flood.

4.5 Risk zoning results



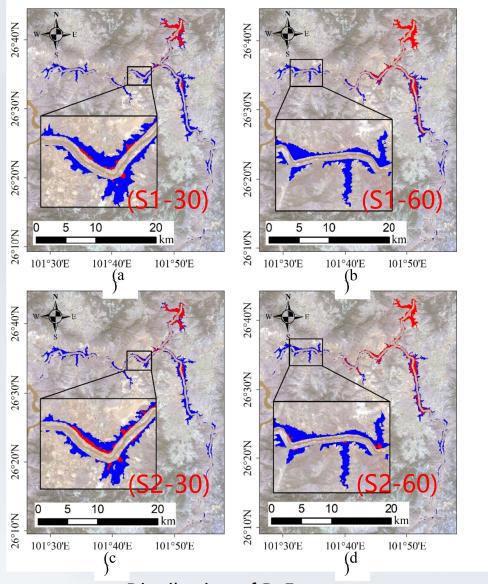


related to the flood arrival time.
There were no apparent
differences between the Yalong
River and downstream of the
Jinsha River. However, in the
upstream of the Jinsha River,
the flood arrival earlier in S2
than in S1. The different floodwave velocities in the upstream
of the Jinsha River were
responsible for this difference.

Risk zoning is strongly

4.6 Risk zoning results





Dead zone

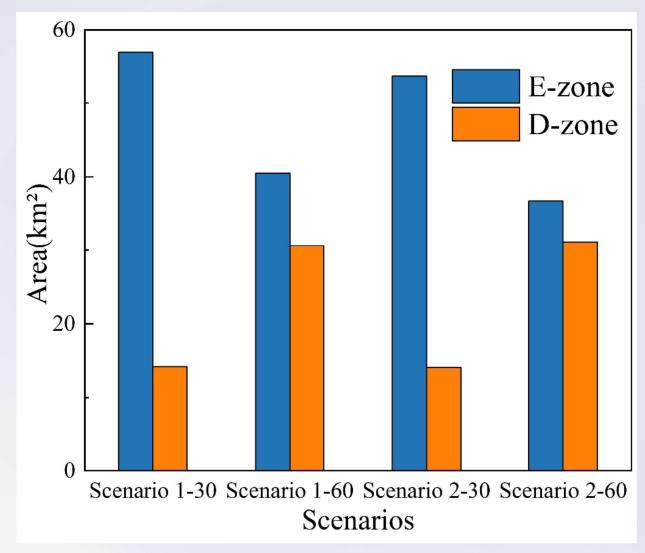
Escape zone

As expected, the disparities between the scenarios are most prominently observed in the upper Jinsha River. In particular, the Bingcaogang Bridge was not included in the D-zone when the EGT was set to 30 min. However, Bingcaogang Bridge was included in the D-zone when the EGT was increased to 60 min.

Distribution of D-E zone

4.5 Risk zoning results



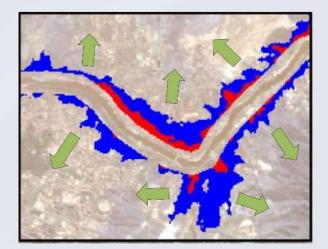


The area of D-E zone

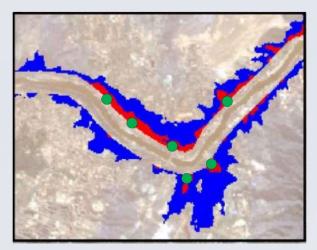
The area of D-zone increases as the EGT increases. When the EGT increased in S1, the **D-zone** expanded by 16.47 km² or **116.12%.** Similar trends may be observed in S2, where there is a 17.04 km² increase (120.81%) between S2-60 and S2-30. Additionally, the D-zone expands by **0.49 km2** between S1-60 and S2-60. This could due to the early arrival of the flood.

4.7 Possible disaster reduction methods

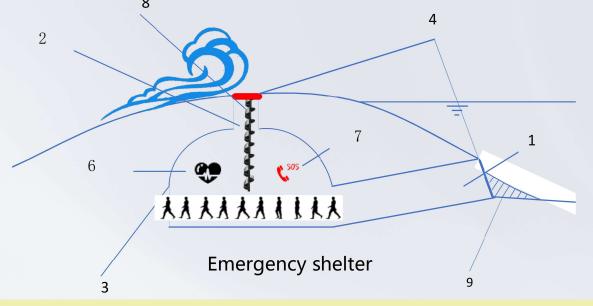




Direction of evacuation

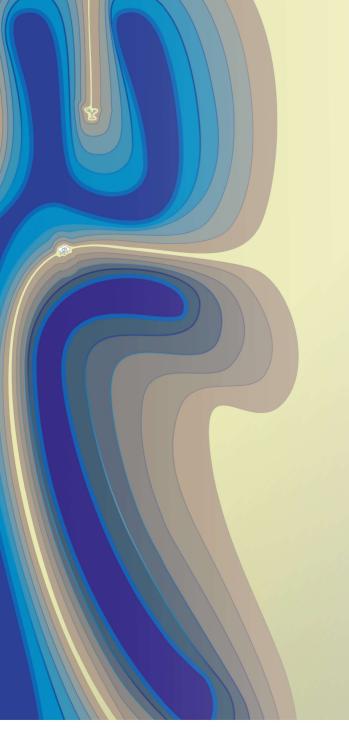


Location of emergency shelter



- 1-Inlet,
- 2-Outlet,
- 3-Shelter,
- 4-Watertight doors,
- 5-Alarm device,
- 6-Life support system,
- 7-Rescue equipment,
- 8-Staircase,
- 9-Silt。

A floodplain can be divided into several zones based on the risks zoning method, allowing for targeted flood control and mitigation measures. In addition, by providing useful data about the distribution of flood risk, this strategy may assist in designing and building projects to mitigate floods. For example, planning transfer routes and building emergency shelters.





Content

- Introduction
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- Summarize

5 Summarize



- A new coupling model was developed to couple river networks and surface flows.
- A **20 min reduction** in the duration of the reverse flow is caused by the flood inflow (1000-year return period) of the Jinsha River.
- When the evaluation generation time (EGT) increased from 30 min to 60 min, the D-zone dramatically increased in both flood encounter scenarios.



- The lack of recorded datasets of extremely huge floods led to incomplete model validation.
- The research does not consider the coupling impact between floods and buildings.
- More comprehensive transfer plan need be considered to obtain more accurate zoning results.



